

### LBTGC: Development of new design calculations in consideration of long-term behaviour

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#### Abstract

The research deals with the long-term behavior of bonded timber-glass-composite shear walls under constant shear stress on the basis of theoretical and experimental investigations. Specimens in two different configurations were produced and examined under two different continuous load levels for twelve months and cycling load in question of the shear strength of the adhesive. The small specimens were assembled according to previous examinations in order to compare the results. The medium sized specimens were designed as a shear wall and they were investigated under the same experiments as the small specimens. Only the investigations under cycling load were omitted. The creep deformation of both kinds of specimens was documented and compared. The higher load level with  $0.05 \text{ N/mm}^2$  showed higher creep deformations as the lower load level with  $0.04 \text{ N/mm}^2$ . But under both load levels the creep deformation continued after six months of continuous stress. In previous researches the acceptable load level with no creep deformation after 90 days was defined with  $0.04 \text{ N/mm}^2$ . This conclusion could not be confirmed at the current observation. A difference between the two types of specimens occurred as well. The influence of climatic conditions on the behaviour of the specimens was observed too. These results were used to derive a modification factor for calculating load-bearing timber-glass composite structures. The shear strength tests were held after variable times of preloading to examine the loss of shear strength in order to derive the modulus of shear deformation and deformation factors. The number of ten small specimens for one test series with a load level of  $0.04 \text{ N/mm}^2$  for preloading showed a continuous loss of strength. For the higher load level only five small specimens were tested each time, these showed a high variation of shear strength. The examinations with the medium sized specimens were held with four specimens per load level per test series. Subsequently the achieved results are compared with previous studies and safety factors are derived for the calculation of timber-glass-composite structures under long-term stress according to the same theoretical considerations as used in previous researches. The known coefficients could partially be confirmed but they should be considered critically.

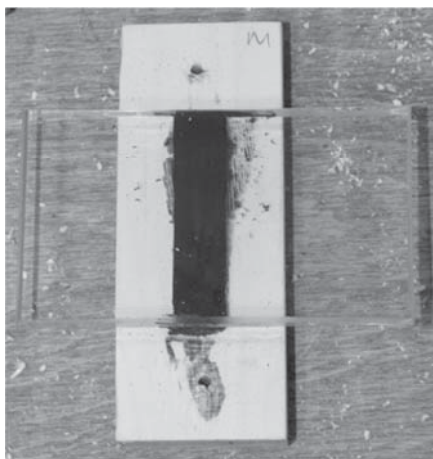


Figure 1: Small specimen

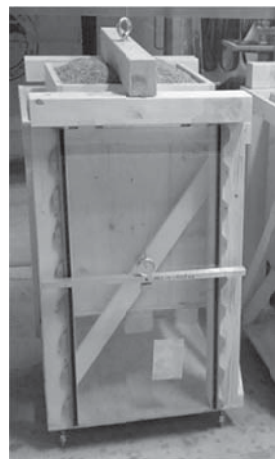


Figure 2: Medium sized specimen



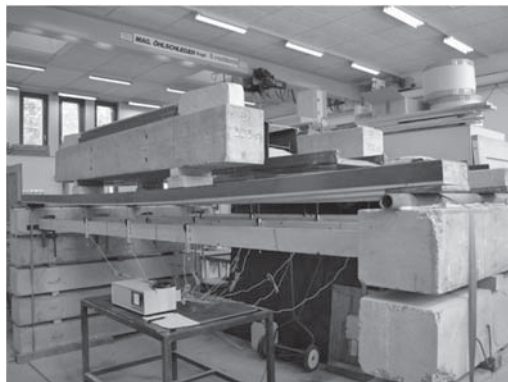
# Development of design factors in consideration of long-term behaviour

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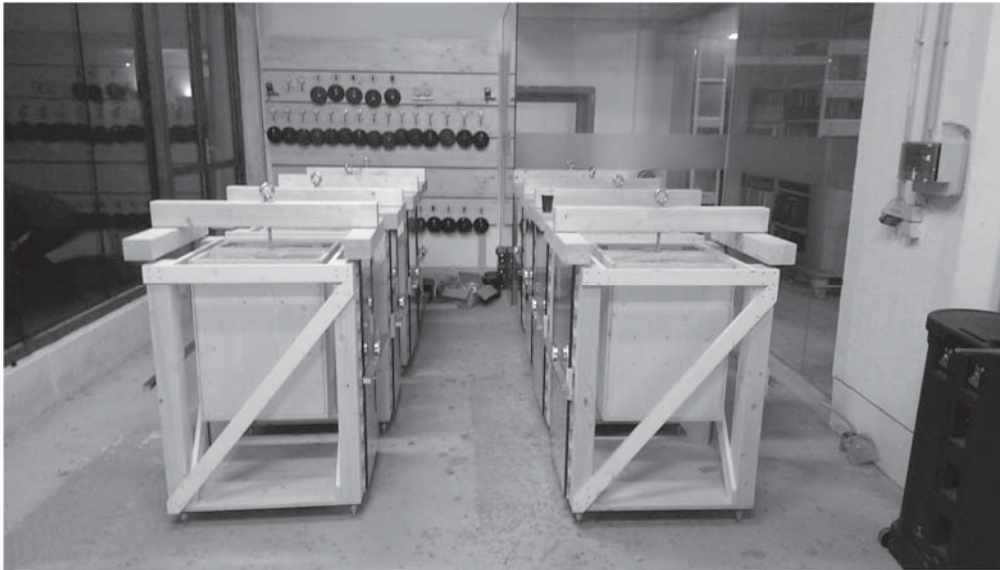
Düsseldorf,  
20.10.2014

 Institute of Architectural Sciences  
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## Long-term behaviour of hybrid constructions



# Long-term behaviour of hybrid constructions



## ETAG 002-1

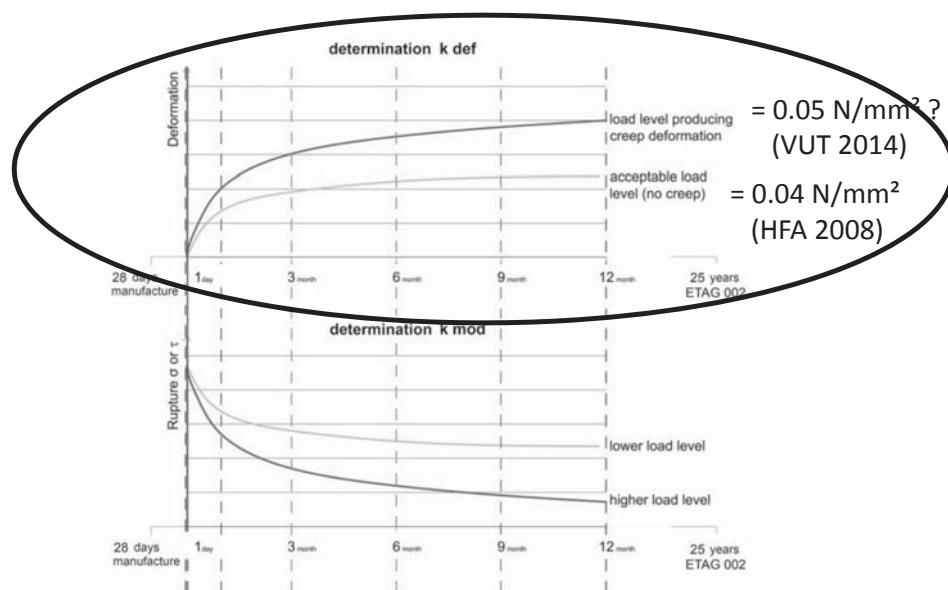
- Guideline for structural sealant glazing systems
- Total safety factor for long-term behaviour =  $1/60$ 
  - ➔ Identify reserves to make LBTGC more attractive

## HFA 2008

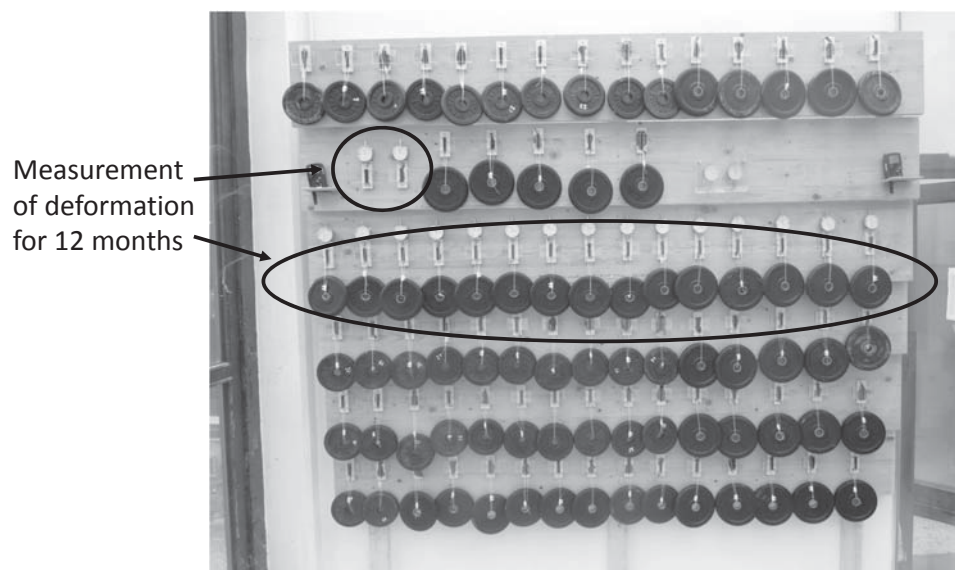
- Experiments to study long-term behaviour of LBTGC elements
- Total safety factor for long-term behaviour =  $1/15$ 
  - ➔ To risky? Still reserves?



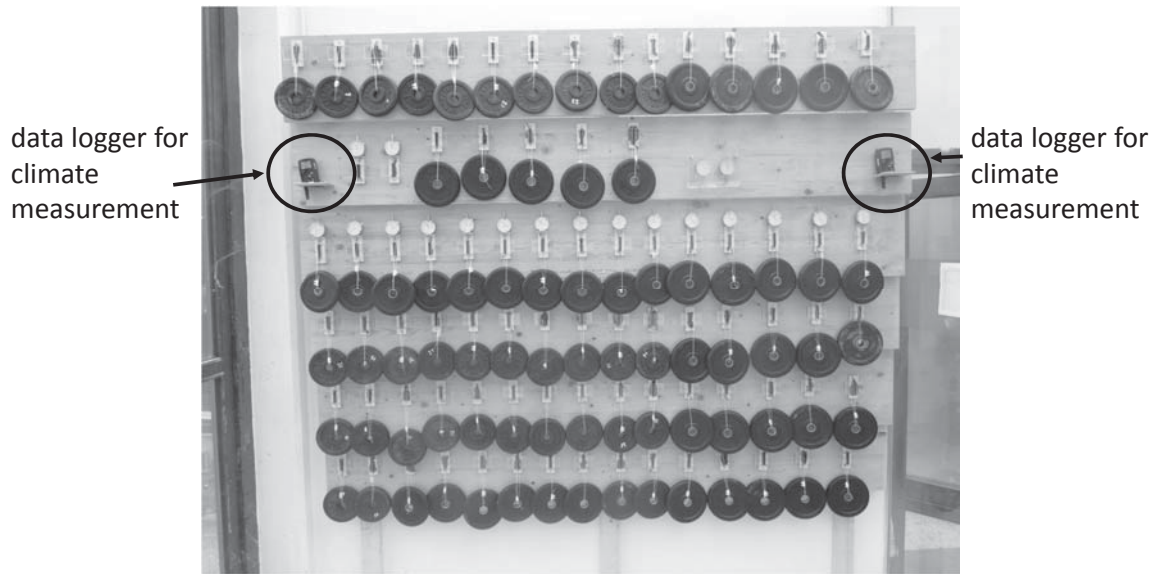
# Modification and deformation factors for LBTGC - elements



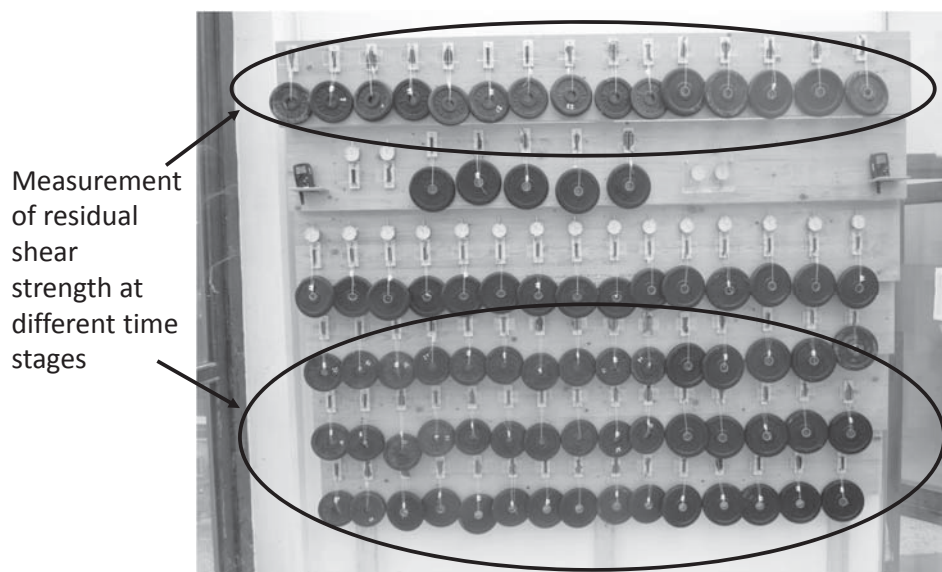
## Small specimens – test setup



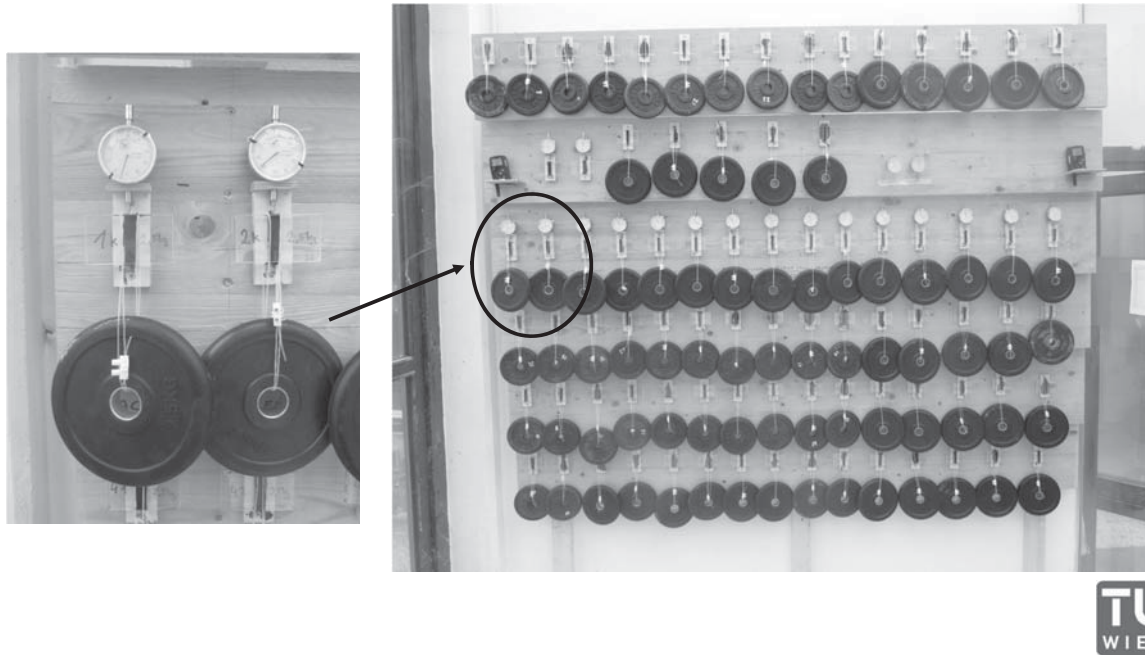
# Small specimens – test setup



# Small specimens – test setup



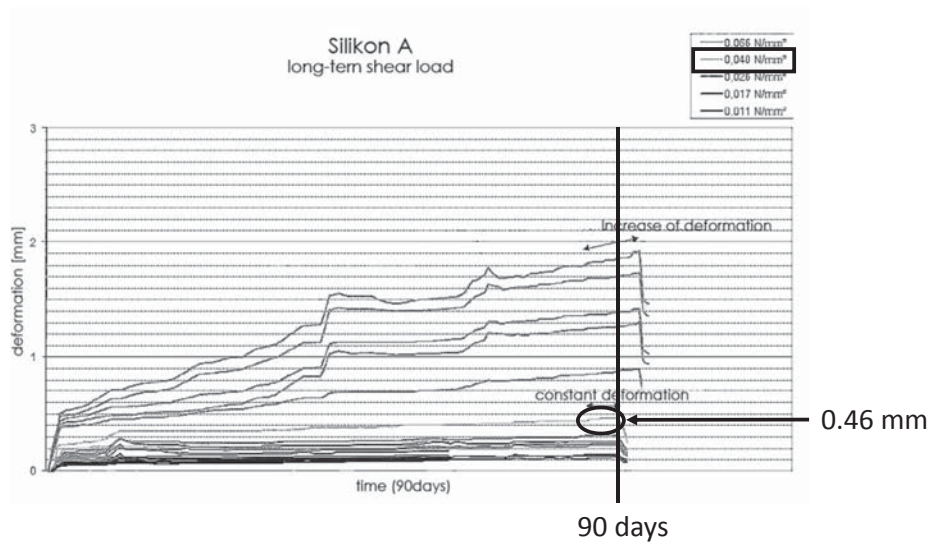
# Small specimens – test setup



# Previous work from HFA 2008

Climatic conditions:

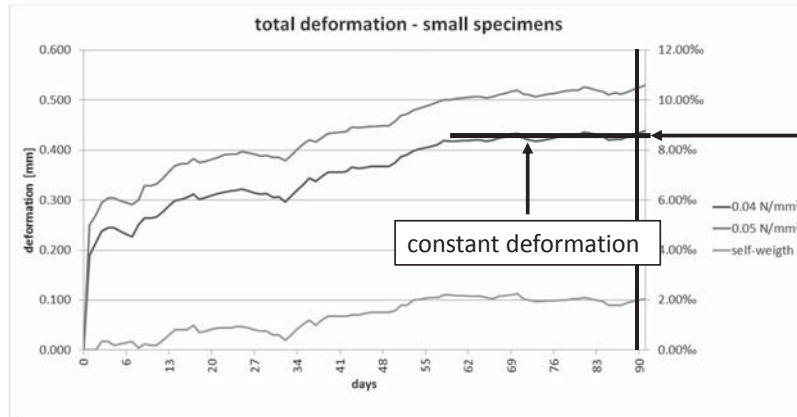
- Temperature = 20 °C
- Relative air humidity = 65 %



# TU Vienna 2014 – 90 days

Climatic conditions:

- Natural indoor climate



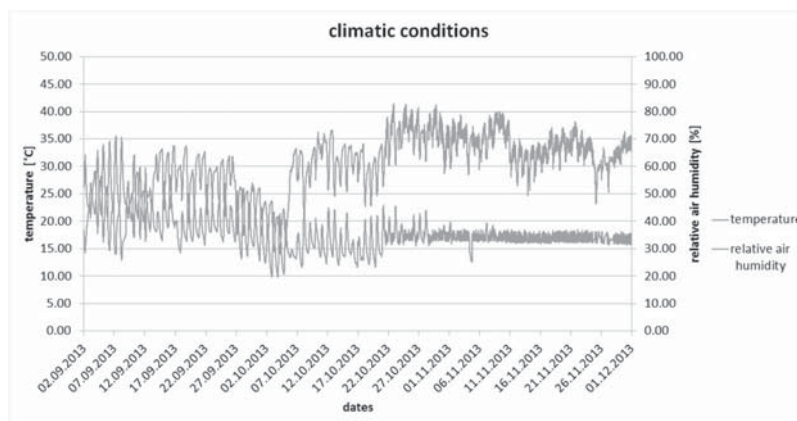
0.435 mm  
(HFA: 0.46 mm)



# TU Vienna 2014 – 90 days

Relative air humidity:

- Maximum: 83.20 %
- Minimum: 25.60 %



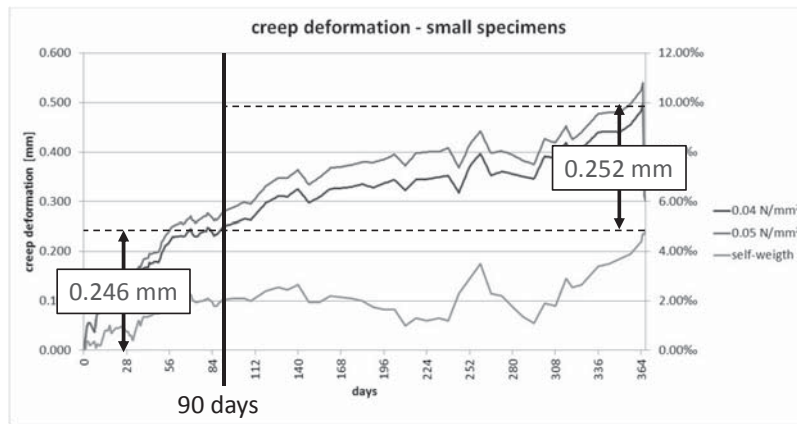
Temperature:

- Maximum: 35.60 °C
- Minimum: 9.60 °C

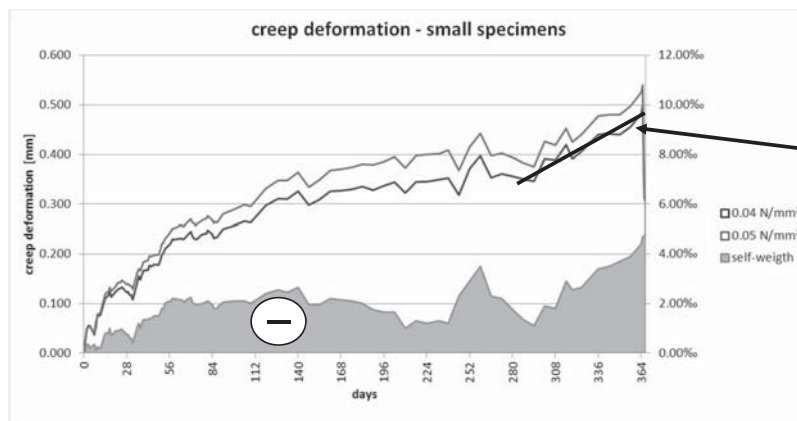




# TU Vienna 2014 – 365 days



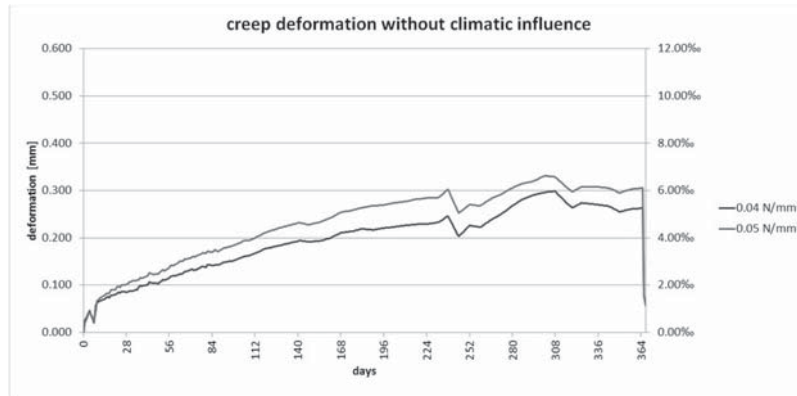
# TU Vienna 2014 – 365 days



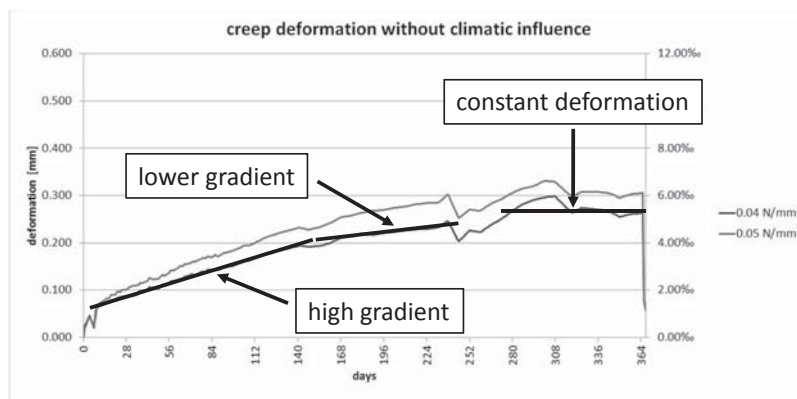
Deformation still increasing!



# TU Vienna 2014 – 365 days



# TU Vienna 2014 – 365 days



# Determination of $k_{def}$

- $k_{def} = \frac{w_{91}}{w_0} - 1$

- $w_{91}$  ... deformation after 91 loaded days
- $w_0$  ... deformation after initial loading

- HFA 2008:

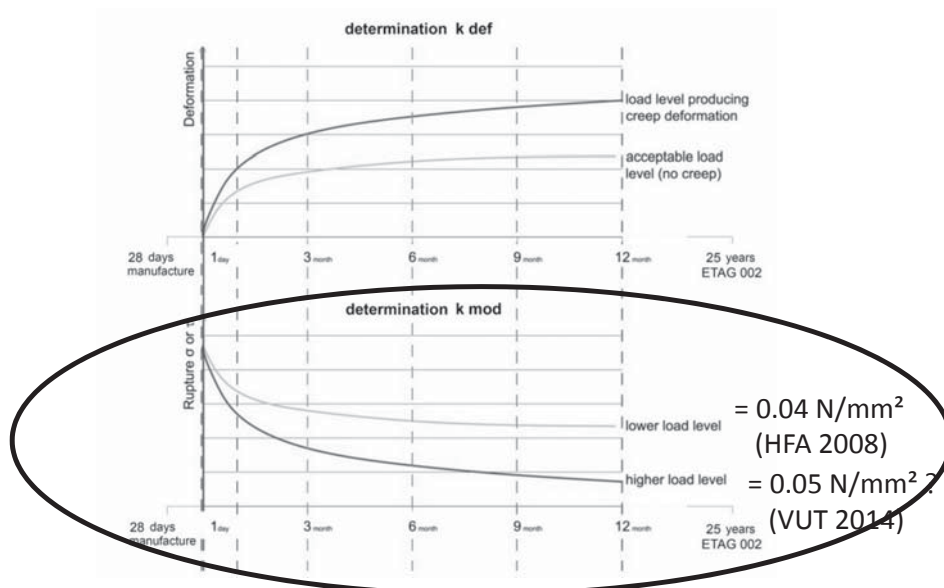
- $k_{def,1} = 1.0$   
(20 °C and 65 % relative air humidity)
- $k_{def,2} = 1.6$   
(55 °C and 65 % relative air humidity)

- VUT 2014:

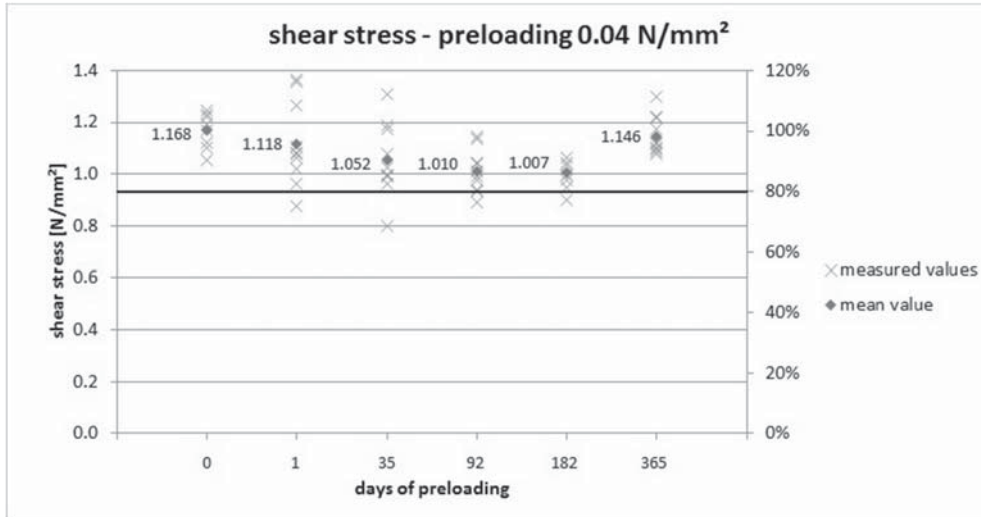
- $k_{def} = \frac{0.435}{0.189} - 1 = 1.30$   
(natural indoor climate)



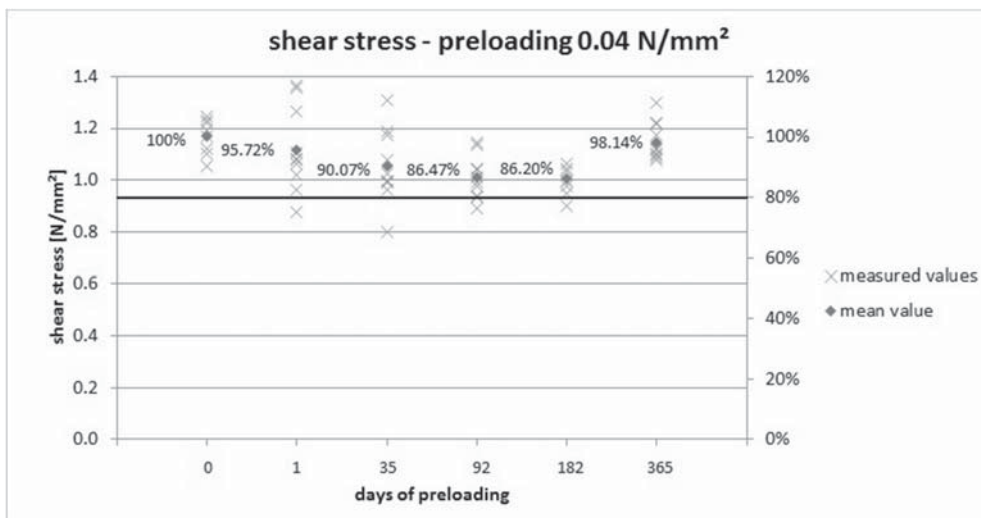
## Modification and deformation factors for LBTGC - elements



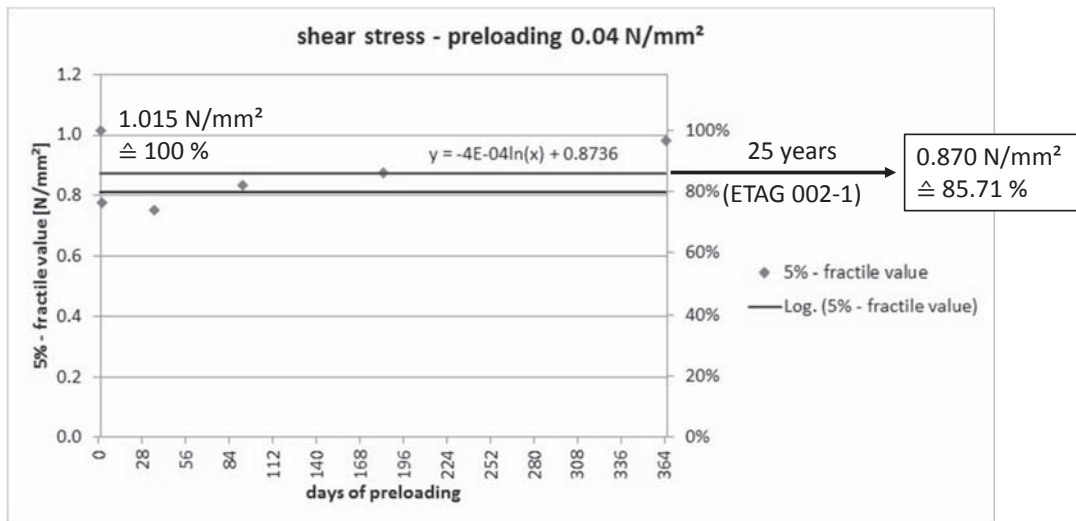
# Residual shear stresses – mean values



# Residual shear stresses – mean values



# Residual shear stresses – 5% fractile value



## Determination of $k_{mod}$ and $\gamma_M$

Theoretical:

$$k_{mod,long} = \frac{\tau_{k,\infty}}{\tau_k}$$

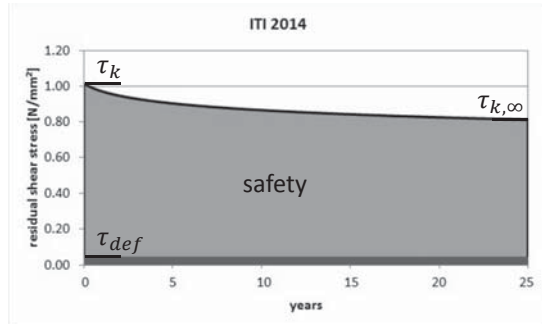
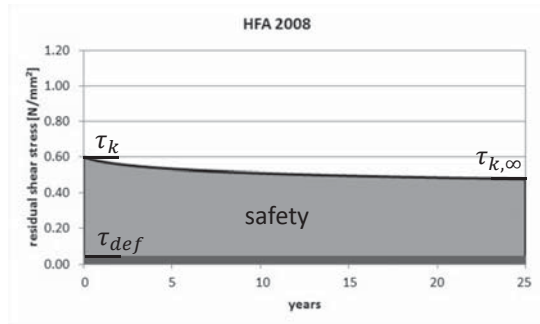
$$k_{mod,long} = \frac{0.870}{1.015} = 0.86$$

VUT 2011:  $\frac{\tau_{k,91+1}}{\tau_k} > 0.8 \quad \checkmark$

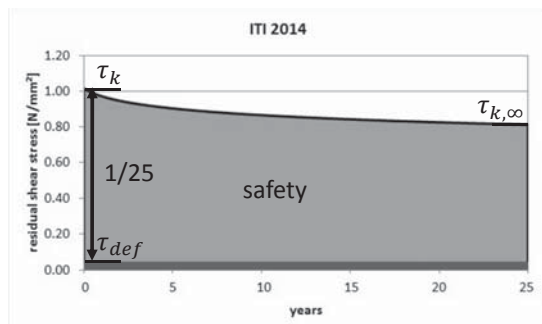
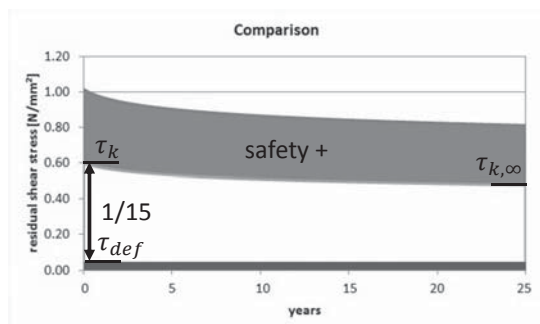
VUT 2014:  $\frac{\tau_{k,\infty}}{\tau_k} > 0.8 \quad \checkmark$



# Safety factors



# Safety factors



$$\frac{\tau_{def}}{\tau_k} = \frac{k_{mod}}{\gamma_M} \rightarrow \frac{0.04}{0.6} = \frac{1}{15}$$

$$\frac{\tau_{def}}{\tau_k} = \frac{k_{mod}}{\gamma_M} \rightarrow \frac{0.04}{1.015} \approx \frac{1}{25}$$



# Determination of $k_{mod}$ and $\gamma_M$

Base:  $\frac{k_{mod}}{\gamma_M} = \frac{1}{25}$

1.  $\gamma_M$  according to VUT 2011:      2.  $k_{mod}$  according to VUT 2011:

- $\gamma_M = 6$       (VUT 2011)
  - $k_{mod} = 0.24$
- $k_{mod} = 0.2$       (VUT 2011)
  - $\gamma_M = 5$

Comparison:			
HFA 2008:	$k_{mod} = 0.2$ $\gamma_M = 3$		→ $\frac{k_{mod}}{\gamma_M} = \frac{1}{15}$
VUT 2011:	$k_{mod} = 0.2$ $\gamma_M = 6$		→ $\frac{k_{mod}}{\gamma_M} = \frac{1}{30}$
ETAG 002-1:	$k_{mod} = 0.1$ $\gamma_M = 6$		→ $\frac{k_{mod}}{\gamma_M} = \frac{1}{60}$



## Outlook

$$\frac{\tau_{def}}{\tau_k} = \frac{k_{mod}}{\gamma_M}$$

- Increase  $\tau_{def}$   
→ 0.05 N/mm<sup>2</sup> as acceptable load-level?
- Decrease  $\tau_k$   
→ why not  $\tau_{k,\infty}$  instead of  $\tau_k$ ?
- Decrease  $\gamma_M$   
→ optimize bondline quality

**Make LBTGC elements  
more economic and more attractive  
for industry and for users!**

